

BAB V

ANALISA PERHITUNGAN

5.1 Data Peninjauan Konstruksi

Data perencanaan kerja proyek pada P.203 Jalan Tol Jakarta–Cikampek II *Elevated* ini meliputi data konstruksi dan data tanah. Data tersebut adalah sebagai berikut:

5.1.1 Data Konstruksi

1. Data Struktur Atas

Bentang antar pilar	: 60 m
Lebar <i>Pier head</i>	: 18,05 m
Panjang <i>Steel Box Girder</i>	: 60 m
Lalu lintas jalan	: 2 Jalur 4 lajur
Tinggi pilar	: beton prategang
Tinggi pilar	: 10,414 m

2. Data Struktur Bawah

Panjang <i>Bore Pile</i>	: 40,5 m
Diameter <i>Bore Pile</i>	: 1,2 m
Jenis konstruksi	: beton bertulang
Berat jenis beton bertulang (γ)	: 2,5 t/m ²
Mutu beton borepile (f _{c'})	: 30 Mpa
Mutu baja tulangan (f _y)	: 392 Mpa

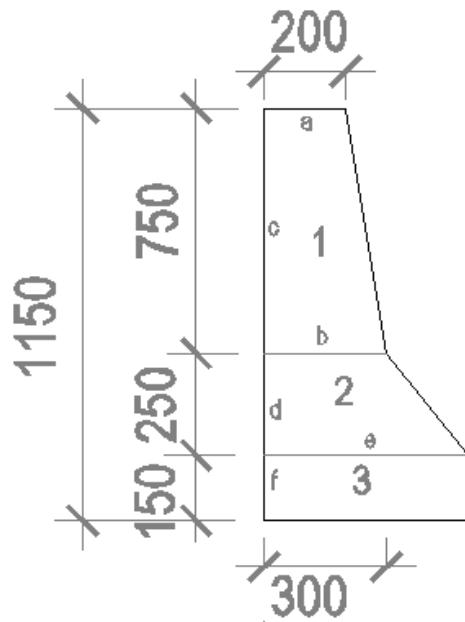
3. Data Tanah

Standart Penetration Test (SPT) : Terlampir

5.2 Perhitungan Beban Konstruksi

5.2.1 Beban Mati

1. Berat Barier



Gambar 5.1 Barier

$$\text{Bentang} = 60 \text{ m}$$

$$\text{Berat jenis beton } (\gamma) = 2,5 \text{ t/m}^3$$

$$\text{Jumlah Barier} = 4 \text{ buah}$$

$$\text{Luas 1} = \frac{1}{2} \times (a + b) \times c$$

$$= \frac{1}{2} \times (0,2 + 0,3) \times 0,75$$

$$= 0,188 \text{ m}^2$$

$$\text{Luas 2} = \frac{1}{2} \times (b + d) \times e$$

$$= \frac{1}{2} \times (0,3 + 0,5) \times 0,25$$

$$= 0,1 \text{ m}^2$$

Luas 3 = $d \times f$

$$= 0,5 \times 0,15$$

$$= 0,075 \text{ m}^2$$

Volume = ($\text{Luas } 1 + \text{Luas } 2 + \text{Luas } 3$) \times Bentang

$$= (0,188 + 0,1 + 0,075) \times 60$$

$$= 0,363 \times 60$$

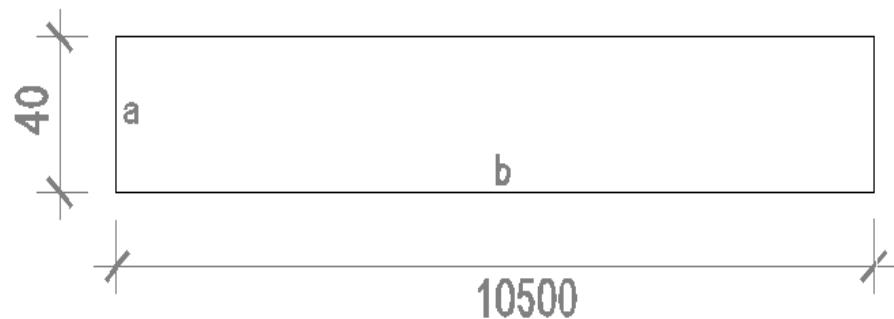
$$= 21,78 \text{ m}^2$$

Berat = Volume $\times \gamma \times$ Jumlah Barier

$$= 21,78 \times 2,5 \times 4$$

$$= 217,8 \text{ t}$$

2. Berat Aspal



Gambar 5.2 Aspal

Bentang = 60 m

Berat Jenis Aspal (γ) = 2,3 t/m³

Luas = $a \times b$

$$= 0,04 \times 10,5$$

$$= 0,42 \text{ m}^2$$

Volume = Luas x bentang

$$= 0,42 \times 60$$

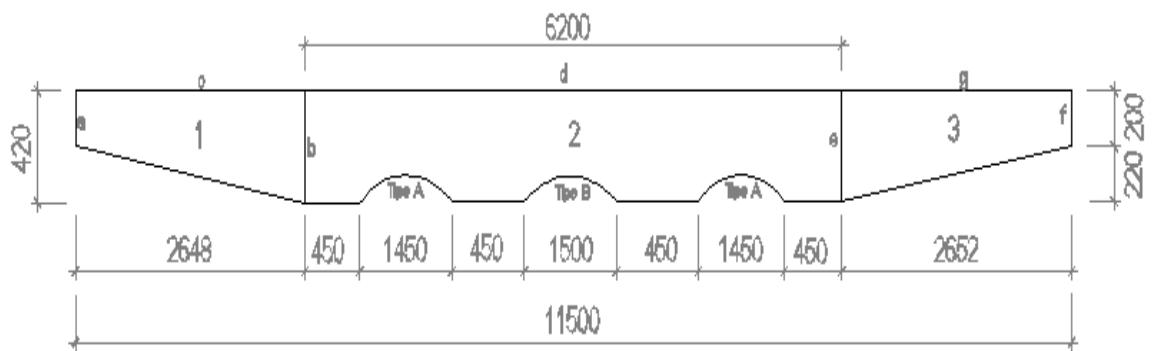
$$= 25,2 \text{ m}^3$$

Berat = Volume x γ

$$= 25,2 \times 2,3$$

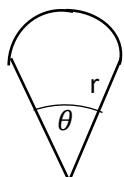
$$= 57,96 \text{ t}$$

3. Berat Slab



Gambar 5.3 Slab

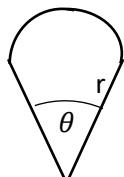
Tipe A



$$r = 1,6995 \text{ m}$$

$$\text{busur} = 1,7285 \text{ m}$$

Tipe B



$$r = 1,8003 \text{ m}$$

$$\text{busur} = 1,7781 \text{ m}$$

Bentang = 60 m

Berat jenis beton (γ) = 2,5 t/m³

$$\text{Luas 1} = \frac{1}{2} \times (a + b) \times c$$

$$= \frac{1}{2} \times (0,2 + 0,42) \times 2,648$$

$$= 0,820 \text{ m}^2$$

Luas 2 = Luas persegi panjang – (2x luas Tembereng Tipe A) – Luas Tembereng
Tipe B

- Luas Persegi Panjang $= b \times d$

$$= 0,4 \times 6,2$$

$$= 2,48 \text{ m}^2$$

- Luas Tembereng Tipe A:

$$\text{Busur} = \frac{\theta}{360^\circ} \times 2\pi r$$

$$\theta = \frac{\text{Busur} \times 360^\circ}{2\pi r}$$

$$\theta = \frac{1,7285 \times 360}{2 \times 3,14 \times 1,6995}$$

$$\theta = 58,303^\circ$$

- Luas Juring $= \frac{\theta}{360^\circ} \times \pi \times r^2$

$$= \frac{58,303}{360^\circ} \times 3,14 \times 1,6995^2$$

$$= 1,469 \text{ m}^2$$

- Luas segitiga = $\frac{1}{2} \times r^2 \times \sin \theta$

$$= \frac{1}{2} \times 1,6995^2 \times \sin 58,303^\circ$$

$$= 1,229 \text{ m}^2$$

- Luas Tembereng Tipe A = Luas juring – luas segitiga

$$= 1,469 - 1,229$$

$$= 0,24 \text{ m}^2$$

- Luas Tembereng Tipe B:

$$\text{Busur} = \frac{\theta}{360^\circ} \times 2\pi r$$

$$\theta = \frac{\text{busur} \times 360}{2\pi r}$$

$$\theta = \frac{1,7781 \times 360}{2 \times 3,14 \times 1,8003}$$

$$\theta = 56,618^\circ$$

$$\text{Luas Juring} = \frac{\theta}{360^\circ} \times \pi \times r^2$$

$$= \frac{56,618}{360^\circ} \times 3,14 \times 1,8003^2$$

$$= 1,601 \text{ m}^2$$

$$\text{Luas segitiga} = \frac{1}{2} \times r^2 \times \sin \theta$$

$$= \frac{1}{2} \times 1,8003^2 \times \sin 56,618^\circ$$

$$= 1,353 \text{ m}^2$$

$$\text{Luas Tembereng Tipe B} = \text{Luas juring} - \text{luas segitiga}$$

$$= 1,601 - 1,353$$

$$= 0,248 \text{ m}^2$$

$$\text{Luas 2} = \text{Luas persegi} - (2 \times \text{Luas Tembereng Tipe A}) - \text{Luas Tembereng Tipe B}$$

$$= 2,48 - (2 \times 0,24) - 0,248$$

$$= 1,752 \text{ m}^2$$

$$\text{Luas 3} = \frac{1}{2} \times (e + f) \times g$$

$$= \frac{1}{2} \times (0,2 + 0,4) \times 2,652$$

$$= 0,796 \text{ m}^2$$

$$\text{Luas Total} = \text{luas 1} + \text{luas 2} + \text{luas 3}$$

$$= 0,820 + 1,752 + 0,796$$

$$= 3,368 \text{ m}^2$$

Volume = luas total x bentang

$$= 3,368 \times 60$$

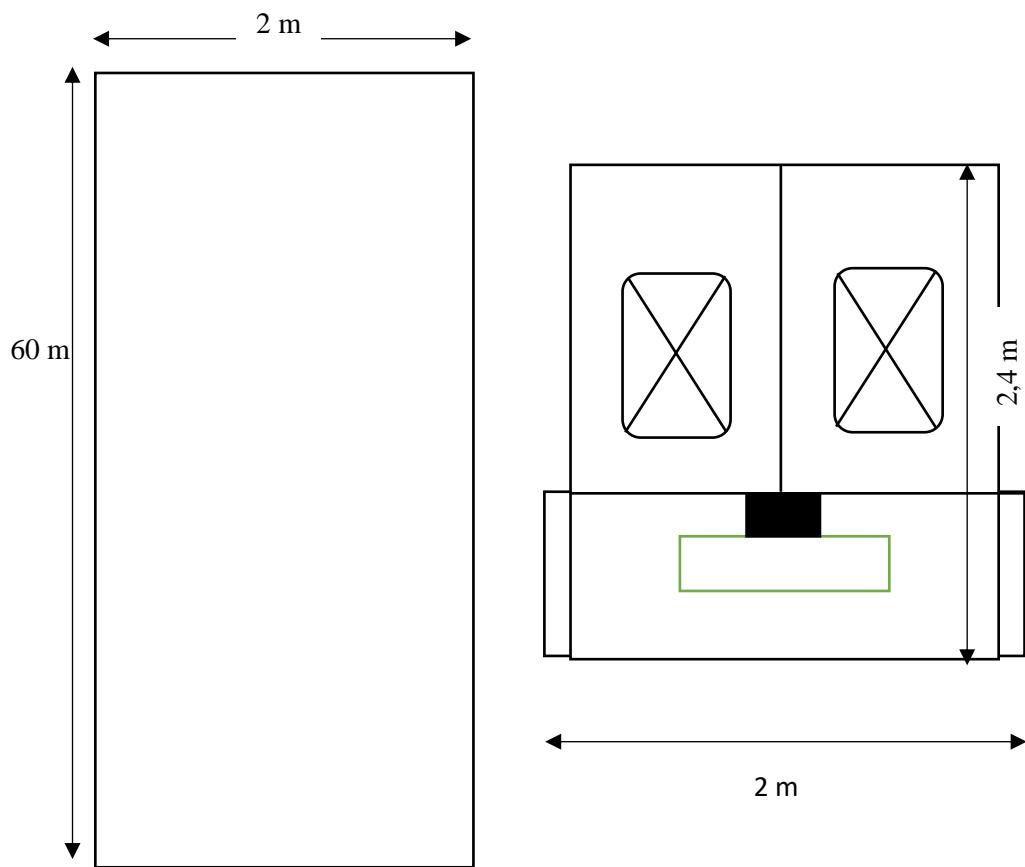
$$= 202,08 \text{ m}^3$$

Berat Slab = Volume x δ x jumlah slab

$$= 202,08 \times 2,5 \times 2$$

$$= 1.010,4 \text{ t}$$

4. Berat Steel Box Girder



Gambar 5.4 Steel Box Girder

Jumlah Girder = 4 buah

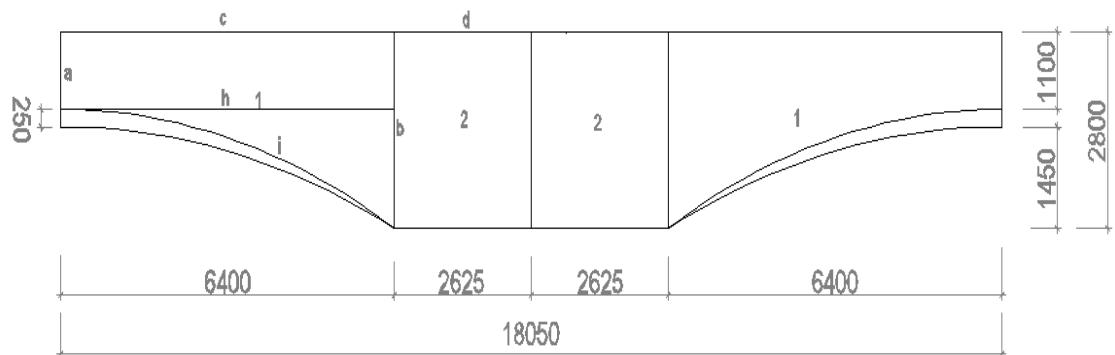
Berat 1 Girder = 75.911,29 kg

$$= 75,911 \text{ t}$$

Berat 2 Girder = $2 \times 75,911$

$$= 151,822 \text{ t}$$

5. Berat Pier Head



Gambar 5.5 Pier Head

Bentang = 3 m

Berat Jenis Beton (γ) = 2,5 t/m³

$$\begin{aligned} \text{Volume 1} &= \left(\frac{1}{2} \times (a + b) \times c \right) \times \text{Bentang} \\ &= \left(\frac{1}{2} \times (1,35 + 2,8) \times 6,4 \right) \times 3 \\ &= 39,84 \text{ m}^3 \end{aligned}$$

$$\begin{aligned} \text{Volume 2} &= b \times d \times \text{Bentang} \\ &= 2,8 \times 2,625 \times 3 \\ &= 22,05 \text{ m}^3 \end{aligned}$$

Panjang i =

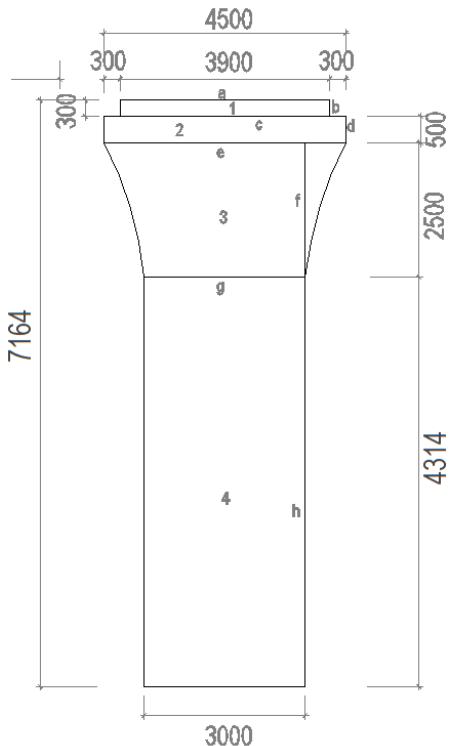
$$\begin{aligned}
 i &= \sqrt{g^2 + h^2} \\
 &= \sqrt{1,7^2 + 6,4^2} \\
 &= \sqrt{43,85} \\
 &= 6,62 \text{ m}
 \end{aligned}$$

$$\begin{aligned}
 \text{Volume 3} &= \left(\frac{1}{2} \times e \times f \right) \times i \\
 &= \left(\frac{1}{2} \times 0,25 \times 1 \right) \times 6,62 \\
 &= 0,828 \text{ m}^2
 \end{aligned}$$

$$\begin{aligned}
 \text{Volume Total} &= (2 \times (\text{volume 1} + \text{volume 2})) - (4 \times \text{volume 3}) \\
 &= (2 \times (39,84 + 22,05)) - (4 \times 0,828) \\
 &= (2 \times 61,89) - (3,312) \\
 &= 123,78 - 3,312 \\
 &= 120,468 \text{ m}^3
 \end{aligned}$$

$$\begin{aligned}
 \text{Berat} &= \text{Volume Total} \times \gamma \\
 &= 120,468 \times 2,5 \\
 &= 301,17 \text{ t}
 \end{aligned}$$

6. Berat Kolom/Pier



Gambar 5.6 Kolom

$$\text{Bentang 1} = 2,5 \text{ m}$$

$$\text{Bentang 2} = 3 \text{ m}$$

$$\text{Berat Jenis Beton } (\gamma) = 2,5 \text{ t/m}^3$$

$$\text{Volume 1} = (a \times b) \times \text{Bentang 1}$$

$$= (3,9 \times 0,3) \times 2,5$$

$$= 2,925 \text{ m}^3$$

$$\text{Volume 2} = (c \times d) \times \text{Bentang 2}$$

$$= (4,5 \times 0,5) \times 3$$

$$= 6,75 \text{ m}^3$$

$$\text{Volume 3} = \left(\frac{1}{2} \times (e + g) \times f \right) \times \text{Bentang 1}$$

$$= \left(\frac{1}{2} \times (4,5 + 3) \times 2,5 \right) \times 2,5$$

$$= 23,438 \text{ m}^3$$

$$\text{Volume 4} = (g \times h) \times \text{Bentang 1}$$

$$= (3 \times 4,314) \times 2,5$$

$$= 32,355 \text{ m}^3$$

$$\text{Volume Total} = \text{Volume 1} + \text{Volume 2} + \text{Volume 3} + \text{Volume 4}$$

$$= 2,925 + 6,75 + 23,438 + 23,438 + 13,605 + 32,355$$

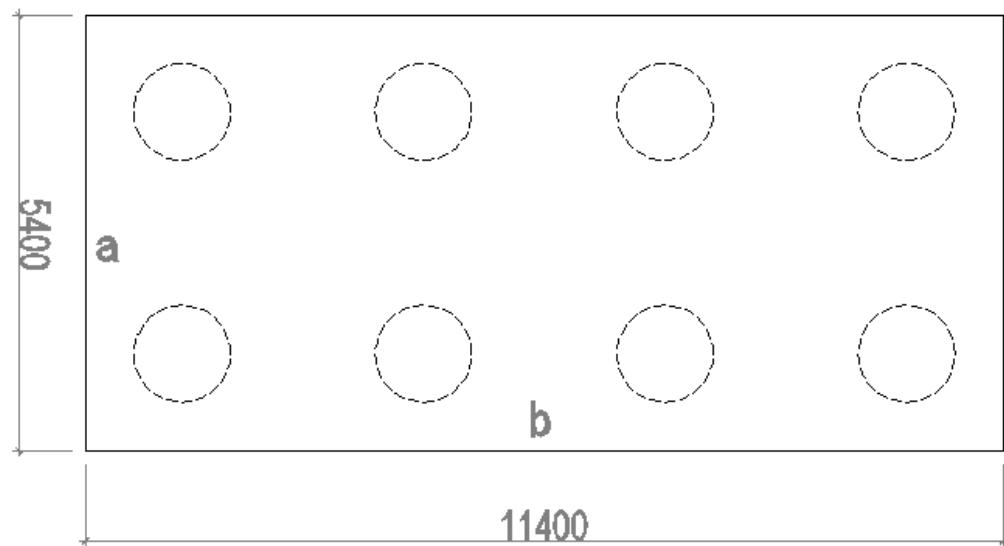
$$= 65,468 \text{ m}^3$$

$$\text{Berat Kolom} = \text{Volume Total} \times \gamma$$

$$= 65,468 \times 2,5$$

$$= 163,67 \text{ t}$$

7. Berat *Pile Cap*



Gambar 5.7 Pile Cap

$$\text{Tinggi } \textit{Pile Cap} = 2,5 \text{ m}$$

$$\text{Berat Jenis Beton } (\gamma) = 2,5 \text{ t/m}^3$$

$$\begin{aligned} \text{Volume Pile Cap} &= a \times b \times \text{bentang } \textit{pile cap} \\ &= 5,4 \times 11,4 \times 2,5 \\ &= 153,9 \text{ m}^3 \end{aligned}$$

$$\begin{aligned} \text{Berat Pile Cap} &= \text{Volume} \times \gamma \\ &= 153,9 \times 2,5 \\ &= 384,75 \text{ t} \end{aligned}$$

Σ Beban Mati:

• Berat <i>Barier</i>	= 217,8 t
• Berat <i>Aspal</i>	= 57,96 t
• Berat <i>Slab</i>	= 1.010,4 t
• Berat <i>Steel Box Girder</i>	= 2 x 151,822 t
• Berat <i>Pier Head</i>	= 301,17 t
• Berat <i>Kolom</i>	= 163,67 t
• Berat <i>Pile Cap</i>	= <u>384,75 t</u> +
	= 2.439,394 t

5.2.2 Beban Hidup

a) Beban “D”

Menghitung menggunakan bentang gelagar

Bentang gelagar (L) = 60 m

Untuk bentang 60 m digunakan rumus :

$$q = 2,2 - \frac{1,1}{60} x (L - 30)$$

maka besar nya q adalahh :

$$q = 2,2 - \frac{1,1}{60} x (L - 30)$$

$$q = 2,2 - \frac{1,1}{60} x (60 - 30)$$

$$q = 2,2 - \frac{1,1}{60} x 30$$

$$q = 2,2 - 0,55$$

$$q = 1,65 t$$

Beban hidup parameter lebar jembatan, dengan rumus:

$$\text{Beban terbagi rata (Q)} = \frac{q}{2,75}$$

$$\text{Beban garis (P)} = \frac{p}{2,75}$$

Untuk beban garis digunakan nilai $P = 12$ (menurut PPPJJR tahun 1987)

$$Q = \frac{q}{2,75} = \frac{1,65}{2,75} = 0,6 \text{ t}$$

$$P = \frac{p}{2,75} = \frac{12}{2,75} = 4,364 \text{ t}$$

Muatan merata diperhitungkan berdasarkan lebar lantai kendaraan.

Lebar lantai jembatan = 11,50 m

Ketentuan PPPJJR 1987:

“ Untuk jembatan dengan lebar lantai kendaraan lebih besar dari 5,50 m, beban “D“ sepenuhnya (100%) dibebankan pada lebar jalur 5,50m. Sedangkan lebar selebihnya dibebani hanya separuh beban “D” (50%)

Sehingga besarnya muatan merata adalah:

$$q = (100\% \times q \times 5,50) + 2 (50\% \times q \times 3,0)$$

$$p = (100\% \times p \times 5,50) + 2 (50\% \times p \times 3,0)$$

dengan berat $Q = 0,6 \text{ t}$ dan $P = 4,364 \text{ T}$

$$q = (100\% \times 0,6 \times 5,50) + 2 (50\% \times 0,6 \times 3,0)$$

$$= 3,3 + 2 (0,9)$$

$$= 3,3 + 1,8$$

$$= 5,1 \text{ t}$$

$$p = (100\% \times 4,364 \times 5,50) + 2 (50\% \times 4,364 \times 3,0)$$

$$= 24,002 + 2 (6,546)$$

$$= 24,002 + 13,092$$

$$= 37,094 \text{ t}$$

$$Q = q \times L$$

$$= 5,1 \times 60$$

$$= 306 \text{ t}$$

b) Beban Kejut

Untuk memperhitungkan beban kejut digunakan beban "P" dan "Q". Untuk beban "P" dikalikan dengan koefisien kejut.

Koefisien kejut dirumuskan:

$$K = 1 + \frac{20}{50+L}$$

dimana:

K = koefisien kejut

L = Panjang bentang

Nilai koefisien kejut sebesar:

$$K = 1 + \frac{20}{50+60}$$

$$= 1 + \frac{20}{110}$$

$$= 1 + 0,182$$

$$= 1,182$$

Untuk beban "P" (beban garis):

$$P = K \times P$$

$$= 1,182 \times 37,094$$

$$= 43,845 \text{ t}$$

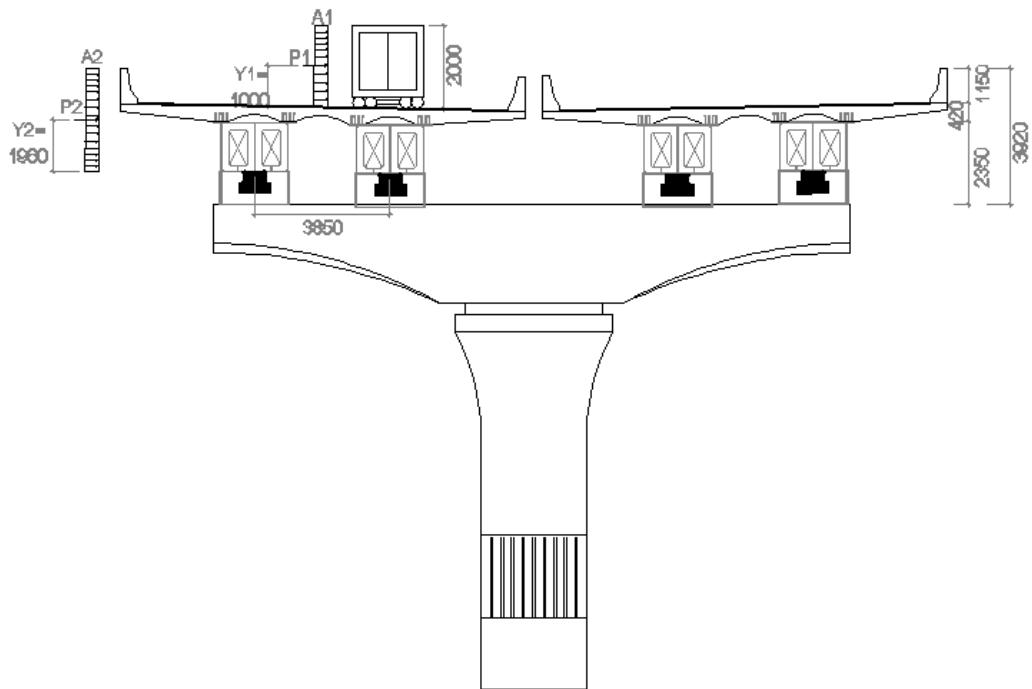
c) Beban Angin

Pengaruh beban angin sebesar 150 kg/m^2 pada jembatan

$$W = 150 \text{ kg/m}^2$$

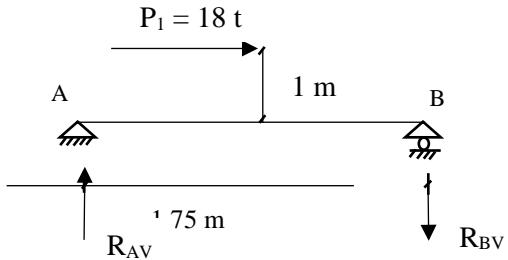
$$W = 0,15 \text{ t/m}^2$$

Bentang = 60 m



Gambar 5.8 Beban Angin

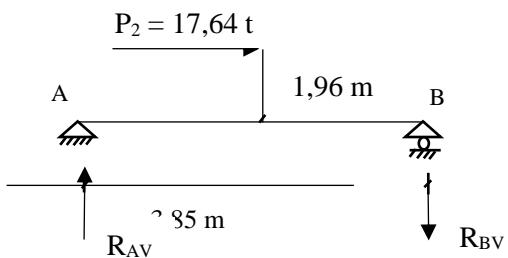
$$\begin{aligned}
 P_1 &= A_1 \times W \times 100\% \\
 &= (2 \times 60) \times 0,15 \times 100\% \\
 &= 120 \times 0,15 \times 100\% \\
 &= 18 \text{ ton}
 \end{aligned}$$



$$\begin{aligned}
 \sum M_B &= 0 \\
 R_{AV} \cdot 1,75 - P_1 \cdot Y_1 &= 0 \\
 R_{AV} \cdot 1,75 - 18 \cdot 1 &= 0 \\
 R_{AV} \cdot 1,75 - 18 &= 0
 \end{aligned}$$

$$R_{AV} = \frac{18}{1,75} = 10,286 \text{ ton}$$

$$\begin{aligned}
 P_2 &= A_2 \times W \times 50\% \\
 &= (3,92 \times 60) \times 0,15 \times 50\% \\
 &= 17,64 \text{ t}
 \end{aligned}$$



$$\begin{aligned}
 \sum M_B &= 0 \\
 R_{AV} \cdot 3,85 - P_2 \cdot Y_2 &= 0 \\
 R_{AV} \cdot 3,85 - 17,64 \cdot 1,96 &= 0 \\
 R_{AV} \cdot 1,75 - 34,574 &= 0
 \end{aligned}$$

$$R_{AV} = \frac{34,574}{3,85}$$

$$R_{AV} = 8,98 \text{ ton}$$

$$\Sigma = 10,286 \text{ t} + 8,98 \text{ t}$$

$$= 19,266 \text{ t}$$

Σ Beban Hidup:

$$\text{Beban D} = 306 \text{ T}$$

$$\text{Beban P} = 43,845 \text{ T}$$

$$\text{Beban Angin} = \underline{19,266 \text{ T}} +$$

$$= 369,111 \text{ T}$$

Σ Beban Vertikal:

$$\text{Beban Mati} = 2.439,394 \text{ T}$$

$$\text{Beban Hidup} = \underline{369,111 \text{ T}} +$$

$$= 2.808,503 \text{ T}$$